# Rigid body equilibrium: Conditions

# Objective:

To introduce the general conditions for 2D and 3D rigid body equilibrium problems.

# Particle equilibrium

First let's recall the conditions for particle equilibrium.

According to **Newton's first law**, an object will have a **linear acceleration of zero** when there is **no unbalanced force** acting on it.

$$\mathbf{F}_R = \sum \mathbf{F} = \mathbf{0}$$

# Particle equilibrium $\mathbf{F}_R = \sum \mathbf{F} = \mathbf{0}$

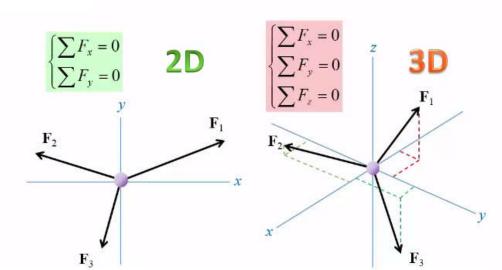
$$\mathbf{F}_R = \sum \mathbf{F} = \mathbf{0}$$

Since Particle is an idealized object with no size or **shape**, and is only represented by **a dot** in space, the forces acting on the particle will be concurrent.

For a 2D problem, the vector equation can be written as 2 scalar equations.

For a 3D problem, the vector equation becomes 3 scalar equations.

2D probem = Solve for 2 unknowns



3D probem = Solve for 3 unknowns

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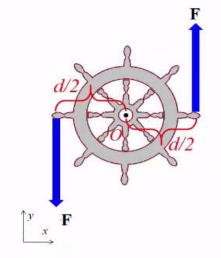
## Recall: moment of a couple

However, a rigid body has shape and size and it is not necessarily static even if the resultant force acting on it is indeed zero.

The two forces acting on this wheel are indeed in equilibrium.

$$\sum F_y = F + (-F) = 0$$

But this only means that they don't cause **translational motion**. We already learned that these two forces make **a couple moment**.



$$\sum M_o = F \cdot \frac{d}{2} + F \cdot \frac{d}{2}$$
$$= F \cdot d$$

This moment causes rotational effect on this wheel.

Therefore, for a rigid body to be static, it is not enough to only have unbalanced force, but the resultant moment summarized about any arbitrary point must be Zero as well.

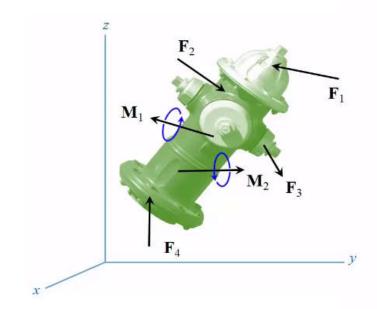
Otherwise, the object will rotate.

For a rigid body that is subjected to multiple forces and couple moments, the first condition for equilibrium is:

$$\mathbf{F}_R = \sum \mathbf{F} = \mathbf{0}$$

Then, the resultant moment summarized about any point, must also be zero, includes both the total moment caused by the forces and the total couple moments.

$$\mathbf{M}_{R,O} = \sum \mathbf{M}_{F,O} + \sum \mathbf{M} = \mathbf{0}$$



## Conditions for rigid body equilibrium

As a summary, for rigid body equilibrium, we can have **two vector equations**, one for force

and one for moment.

$$\begin{cases} \mathbf{F}_{R} = \sum \mathbf{F} = \mathbf{0} \\ \mathbf{M}_{R,O} = \sum \mathbf{M}_{F,O} + \sum \mathbf{M} = \mathbf{0} \end{cases}$$

For a 2D problem, based on one free body diagram, we can write a maximum of 3 independent scalar equations and then solve for 3 unknowns.

2-D problems:

$$\begin{cases} \sum F_x = 0 \\ \sum F_y = 0 \\ \sum M_O = 0 \end{cases}$$

2-D problems:

$$\begin{cases} \sum F_x = 0 \\ \sum M_A = 0 \\ \sum M_B = 0 \end{cases}$$

2-D problems:

$$\begin{cases} \sum M_A = 0 \\ \sum M_B = 0 \\ \sum M_C = 0 \end{cases}$$

# Conditions for rigid body equilibrium

As a summary, for rigid body equilibrium, we can have **two vector equations**, one for force and one for moment.

$$\begin{cases} \mathbf{F}_{R} = \sum \mathbf{F} = \mathbf{0} \\ \mathbf{M}_{R,O} = \sum \mathbf{M}_{F,O} + \sum \mathbf{M} = \mathbf{0} \end{cases}$$

For a 3D problem, based on one free body diagram, we can write a maximum of 6 independent scalar equations and solve for a maximum of 6 unknowns.

#### 3-D problems:

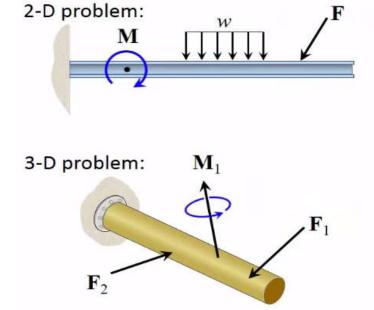
$$\begin{cases} \sum F_x = 0 \\ \sum F_y = 0 \end{cases} \begin{cases} \sum M_x = 0 \\ \sum M_y = 0 \\ \sum M_z = 0 \end{cases}$$

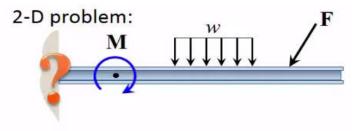
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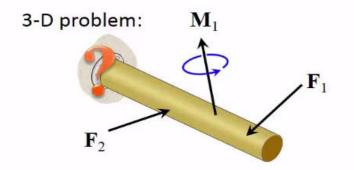
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Here are **two examples** of rigid body **equilibrium problems**. Normally the **applied loadings** are **known**, and we will need to use **the equilibrium equations** to find the **unknown support** reactions.

The support reactions are also external force or moment acting on the body.







# Rigid body equilibrium: 2D Supports

# Objectives:

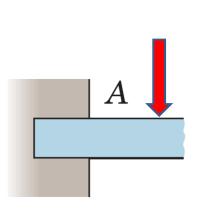
- To explain how to identify 2D support reactions.
- To demonstrate how to solve 2D rigid body equilibrium problems.

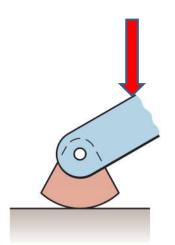
## **Support types:**

• The reaction in the element depends on the support it self.

#### Question:

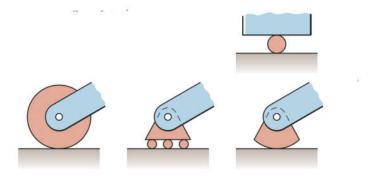
What is the difference here:

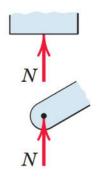




## **Support types:**

Roller Support

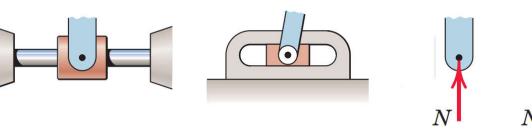




Roller, rocker, or ball support transmits a compressive force normal to the supporting surface.

## **Support types:**

Freely sliding guide



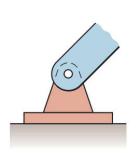


Collar or slider free to move along smooth guides; can support force normal to guide only.

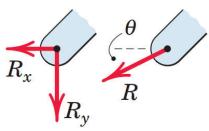
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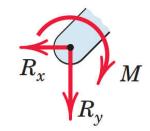
## **Support types:**

Pin connection



Pin free to turn



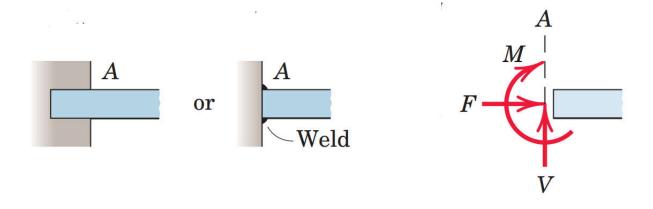


A freely hinged pin connection is capable of supporting a force in any direction in the plane normal to the pin axis. We may either show two Pin not free to turn components  $R_x$  and  $R_{y}$  or a magnitude Rand direction  $\theta$ . A pin not free to turn also supports a couple M.

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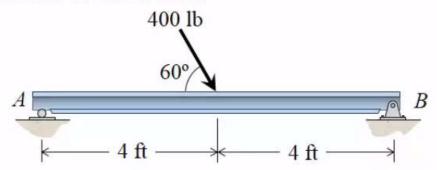
## **Support types:**

Pin connection



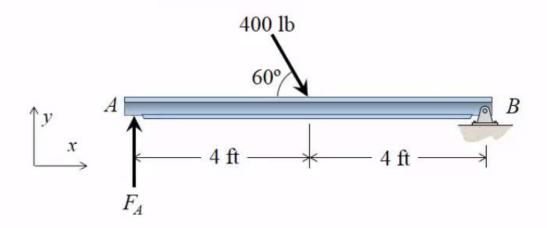
A built-in or fixed support is capable of supporting an axial force F, a transverse force V (shear force), and a couple M (bending moment) to prevent rotation.

Example 3: Determine the support reactions at the roller at A and the pin at B. Neglect the weight and size of the beam.



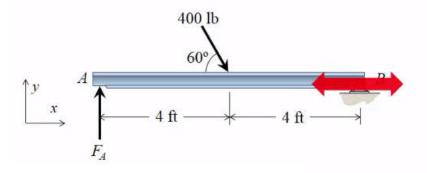
We need to determine the support reactions at the roller and the pin.

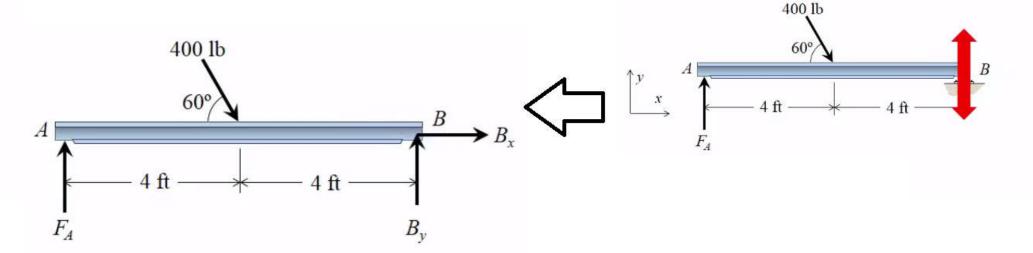
# Free body diagram (FBD)



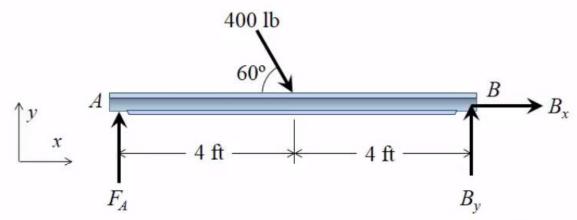
A **roller is similar to a rocker**, or **a simple contact** support. It exerts a force that is perpendicular to the contacting surface.

A pin support allows rotation, but it prevents motion in both **horizontal direction** and the **vertical direction**.





# Free body diagram (FBD)



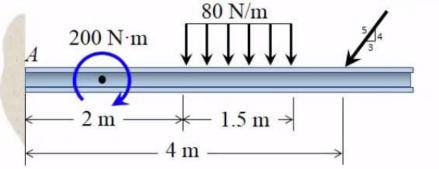
$$\sum F_x = 400 \text{ lb} \cdot \cos 60^\circ + B_x = 0$$

$$\sum F_y = F_A - 400 \text{ lb} \cdot \sin 60^\circ + B_y = 0$$

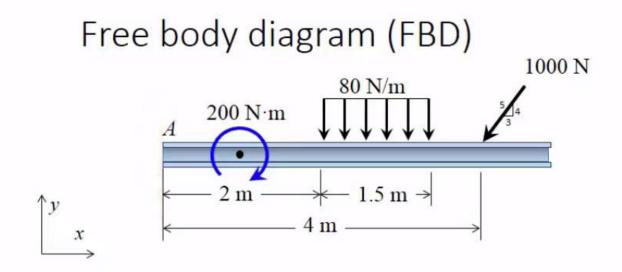
$$\sum M_B = -F_A \cdot 8 \text{ ft} + 400 \text{ lb} \cdot \sin 60^\circ \cdot 4 \text{ ft} = 0$$

$$\therefore \begin{cases} F_A = 173 \text{ lb} \\ B_x = -200 \text{ lb} \\ B_y = 173 \text{ lb} \end{cases}$$

Example 1: Determine the support reactions at the fixed support, A. Neglect the weight and size of the beam.  $1000 \, \mathrm{N}$ 

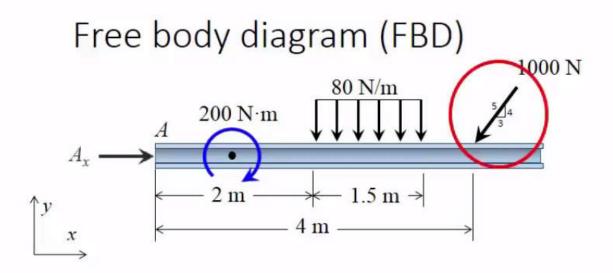


The applied loadings acting on this beam is known and we need to determine the support reactions at the fixed support A.

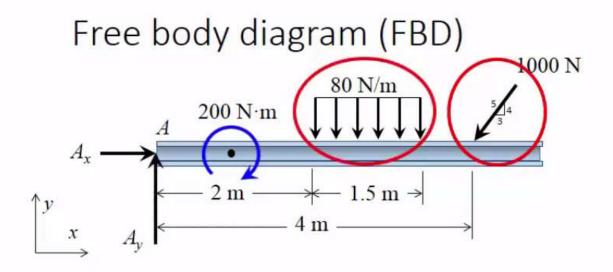


- 1) We need to first isolate our object. Therefore, the wall needs to disappear.
- 2) We need to note all external forces and moments on the diagram.

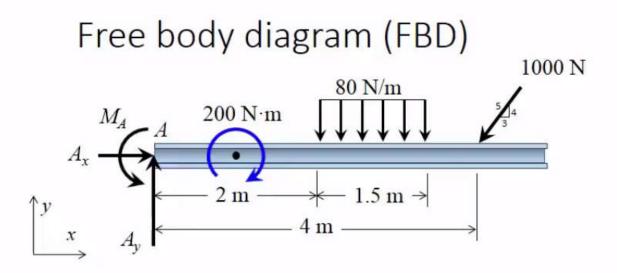
First, we need to be clear that the body is in equilibrium, in other words, it is not moving.



We see that there is a **horizontal component** of this force that will **push** the member to the **left**. Therefore, the wall must exert a horizontal force to the **right** to prevent this motion to the **left**.

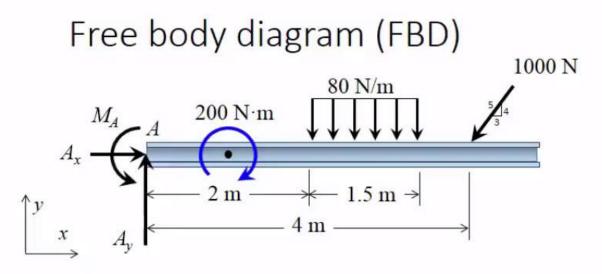


Because of the vertical component of the **concentrated force**, as well as **the distributed force**, causing the member **to go down**. Therefore, the wall must exert **a vertical force up** in order to prevent this downward motion.



Finally, the forces and **the moment** all cause the member to **rotate** clockwise. Therefore, the wall also must exert **a moment support** that is **counterclockwise**, in order to prevent this **rotational motion**.

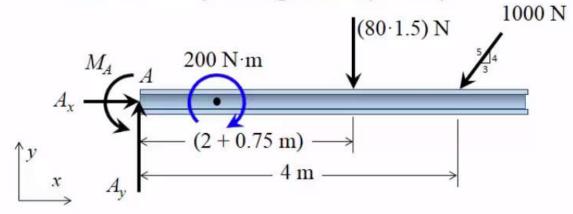
Since this is a **2D problem**, all motions can only accur within the **xy plane**, therefore there can not be any other type of motion, and the wall **will not provide** any other type of **support reactions**.



We have 3 unknowns:  $A_x$ ,  $A_y$  and  $M_A$ .

We can write the three equilibrium equations to solve for all of them.

# Free body diagram (FBD)



$$\sum F_x = A_x - \frac{3}{5} \cdot 1000 \text{N} = 0$$

$$\sum F_y = A_y - 80 \cdot 1.5 \text{N} - \frac{4}{5} \cdot 1000 \text{N} = 0$$

$$\sum M_A = M_A - 200 \text{N} \cdot \text{m} - 80 \cdot 1.5 \text{N} \cdot 2.75 \text{m}$$

$$-\frac{4}{5} \cdot 1000 \text{N} \cdot 4 \text{m} = 0$$

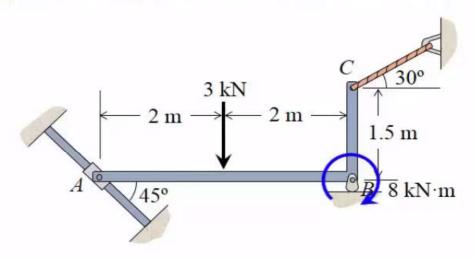
$$\therefore \begin{cases} A_x = 600 \text{ N} \\ A_y = 920 \text{ N} \\ M_A = 3730 \text{ N} \cdot \text{m} \end{cases}$$

## Identify all the support reactions

 If a support prevents translational effect in a direction – exerts a force.

- If a support prevents rotational effect about an axis
  - exerts a couple moment.

Example 2: Determine the support reactions at the smooth collar at A, rocker at B and cable at C. Neglect the weight and size of member ABC.

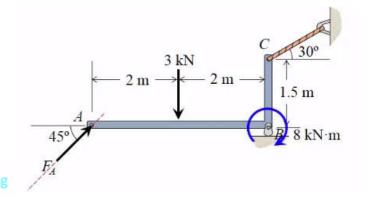


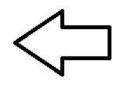
- It is supported by a **smooth collar** at point A.
- A **rocker** at point B.
- A cable at point C.

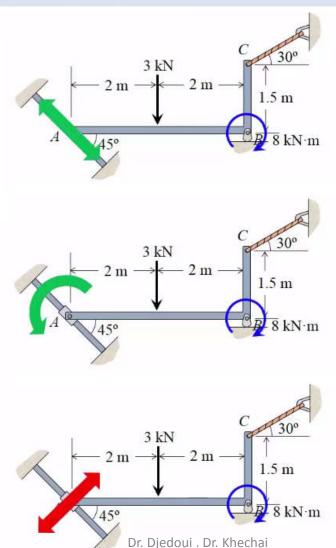
We are asked to determine the support reactions at these points A, B and C.

### Smooth collar:

- 1) It does allow **movement** in this direction.
- 2) Because **the member is pinned** to the **coller**, the member is allowed **to rotate**.
- 3) The only motion **it prevents** is along the direction **perpendicular** to the rod.



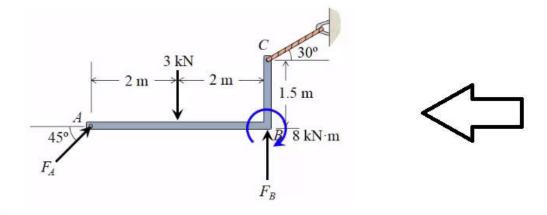


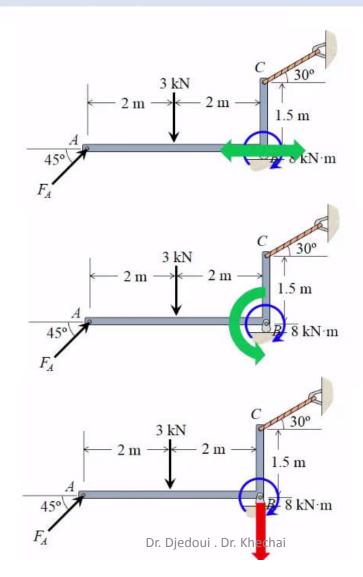


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## **Rocker:**

- 1) It allows motion along the horizontal direction.
- 2) It allows rotations as well.
- 3) The only motion **it does not allow** is for the member **to move down**.

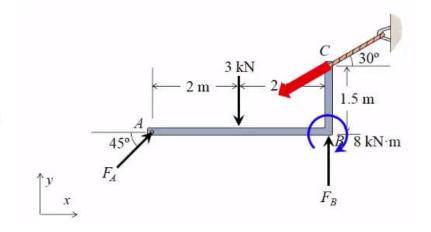


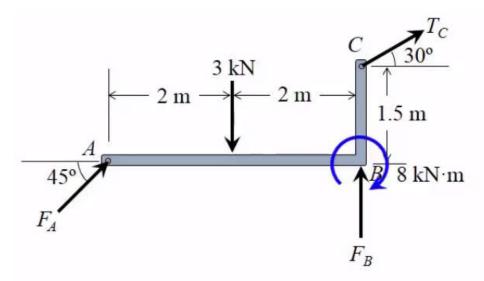


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## Cable:

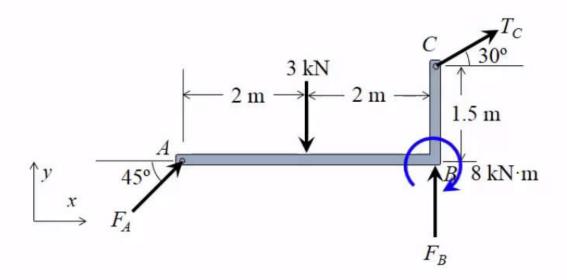
Cable at point C allows all motions except for when the member wants **to move away**.





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$$\sum F_x = F_A \cdot \cos 45^\circ + T_C \cdot \cos 30^\circ = 0$$

$$\sum F_y = F_A \cdot \sin 45^\circ - 3kN + F_B + T_C \cdot \sin 30^\circ = 0$$

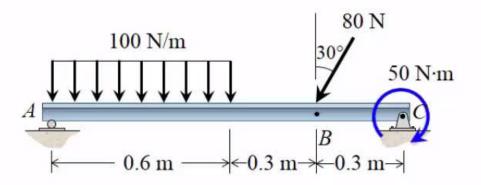
$$\sum M_C = -F_A \cdot \sin 45^\circ \cdot 4m + F_A \cdot \sin 45^\circ \cdot 1.5m$$

$$+ 3kN \cdot 2m - 8kN \cdot m = 0$$

$$F_A = -1.13 \text{ kN}$$

$$T_C = 0.924 \text{ kN}$$

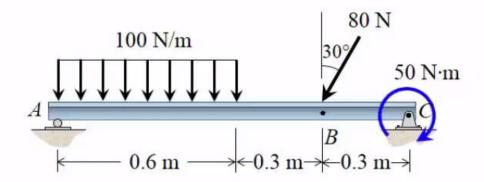
Question 1: For the beam as shown, what is the magnitude of the support force at roller support A?



- (a) 109 N
- (c)  $116 \,\mathrm{N}$

- (b) 40 N
- (d) 20.7 N

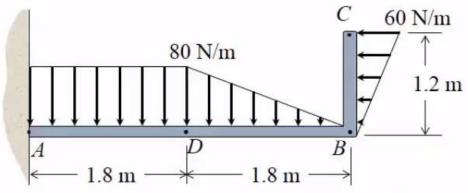
Question 2: For the beam as shown, what is the magnitude of the support force at pin support C?



- (a) 109 N
- (c) 116 N

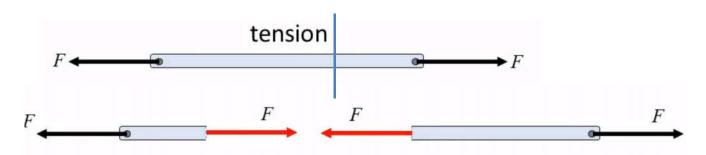
- (b) 40 N
- (d) 20.7 N

Question 3: For the beam as shown, determine the support reactions at the fixed support at point A.

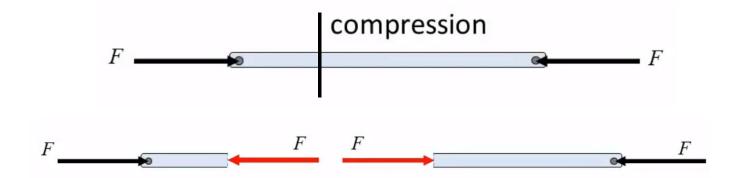


## **Internal forces:**

Method of Section (introduction)



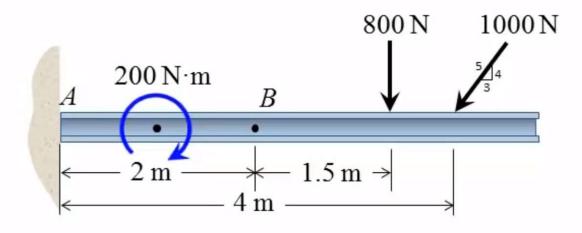
$$\sigma = \frac{F}{A} \le \sigma_{allow}$$



## **Internal forces:**

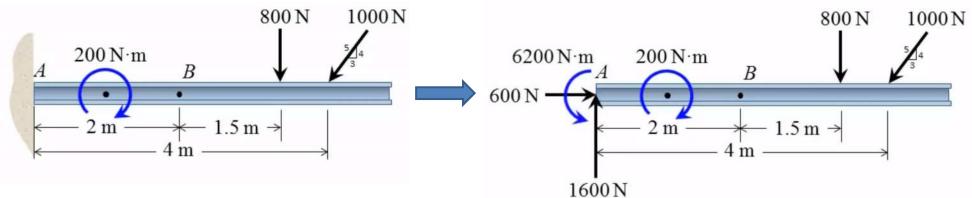
Method of Section: example

Determine the internal forces at point *B* of the cantilever beam.



### **Internal forces:**

Step 1: if necessary, determine the external support reactions(s)

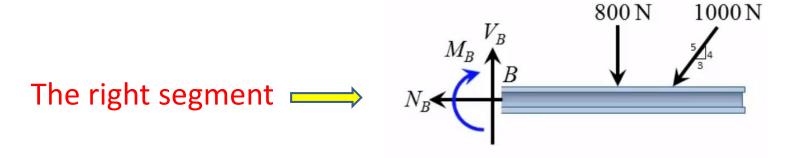


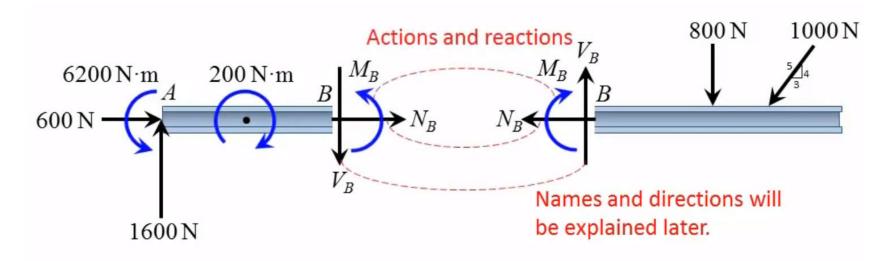
• Step 2: 'cut' the member at the specified point and not the unknowns



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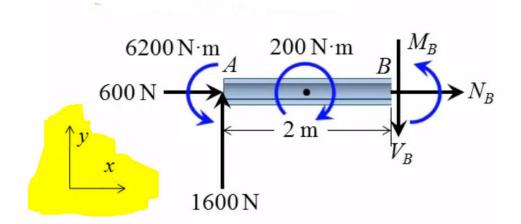
## **Internal forces:**





## **Internal forces:**

### Solving for the left segment:

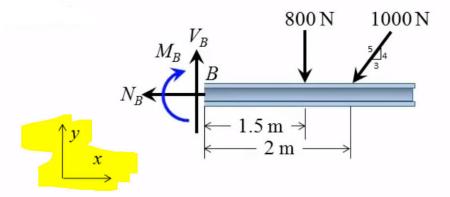


$$\sum F_x = 600 \text{N} + N_B = 0$$
 
$$\sum F_y = 1600 \text{N} - V_B = 0$$
 
$$\sum M_B = 6200 \text{N} \cdot \text{m} - 200 \text{N} \cdot \text{m}$$
 
$$-1600 \text{N} \cdot 2 \text{m} + M_B = 0$$

$$\begin{cases} N_B = -600 \text{ N} \\ V_B = 1600 \text{ N} \\ M_B = -2800 \text{ N} \cdot \text{m} \end{cases}$$

## **Internal forces:**

### Solving for the right segment:



$$\sum F_x = -N_B - \frac{3}{5} \cdot 1000 \,\text{N} = 0$$

$$\sum F_y = V_B - 800 \,\text{N} - \frac{4}{5} \cdot 1000 \,\text{N} = 0$$

$$\sum M_B = -M_B - 800 \,\text{N} \cdot 1.5 \,\text{m}$$

$$\frac{4}{5} \cdot 1000 \,\text{N} \cdot 2 \,\text{m} = 0$$

$$\therefore \begin{cases} N_B = -600 \,\text{N} \\ V_B = 1600 \,\text{N} \\ M_B = -2800 \,\text{N} \cdot \text{m} \end{cases}$$

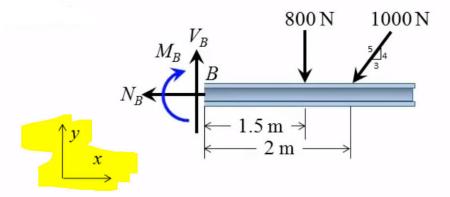
N: Normal Force

V: shear force

M: Bending Moment

## **Internal forces:**

### Solving for the right segment:



$$\sum F_x = -N_B - \frac{3}{5} \cdot 1000 \,\text{N} = 0$$

$$\sum F_y = V_B - 800 \,\text{N} - \frac{4}{5} \cdot 1000 \,\text{N} = 0$$

$$\sum M_B = -M_B - 800 \,\text{N} \cdot 1.5 \,\text{m}$$

$$\frac{4}{5} \cdot 1000 \,\text{N} \cdot 2 \,\text{m} = 0$$

$$\therefore \begin{cases} N_B = -600 \,\text{N} \\ V_B = 1600 \,\text{N} \\ M_B = -2800 \,\text{N} \cdot \text{m} \end{cases}$$

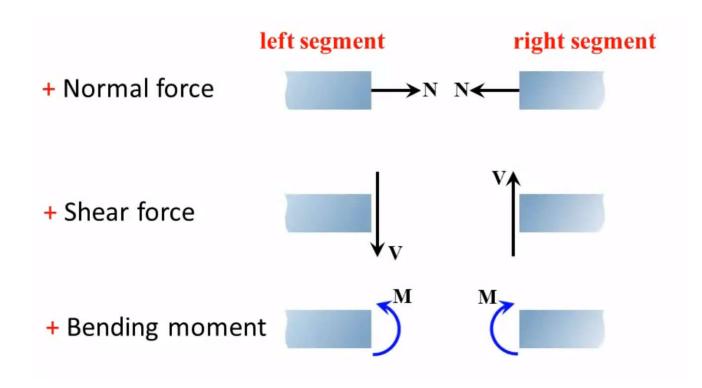
N: Normal Force

V: shear force

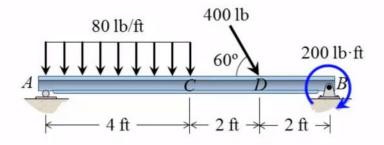
M: Bending Moment

## **Internal forces:**

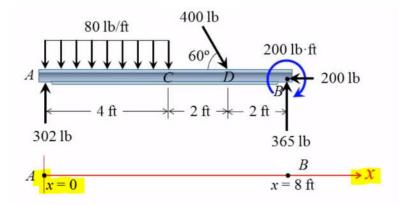
## Sign convention:

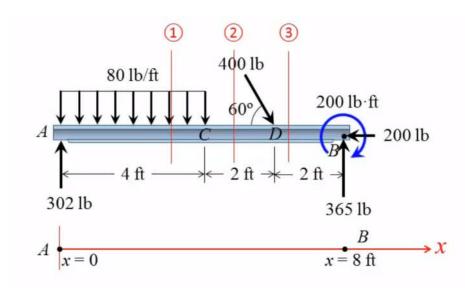


# **Internal forces: Shear fore and bending moment functions and diagrams**



### Free Body diagram (FBD):

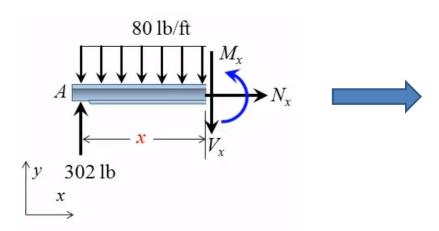


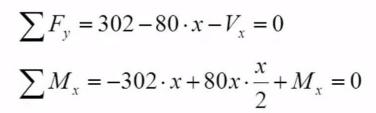


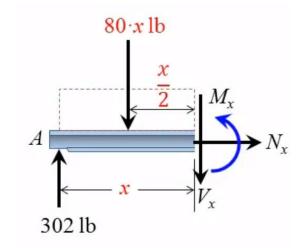
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# Internal forces: Shear fore and bending moment functions and diagrams

#### **Section 1:**



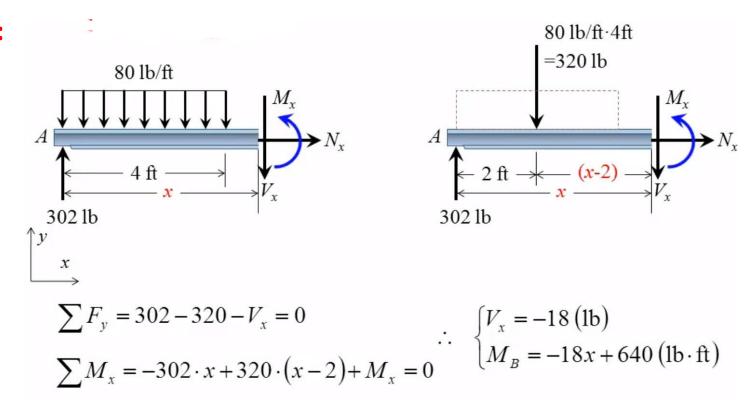




$$\begin{cases} V_x = -80x + 302 \text{ (lb)} \\ M_x = -40x^2 + 302x \text{ (lb · ft)} \end{cases}$$

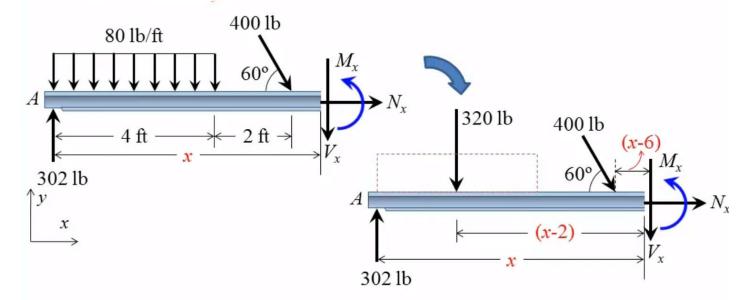
## Internal forces: Shear fore and bending moment functions and diagrams

#### **Section 2:**



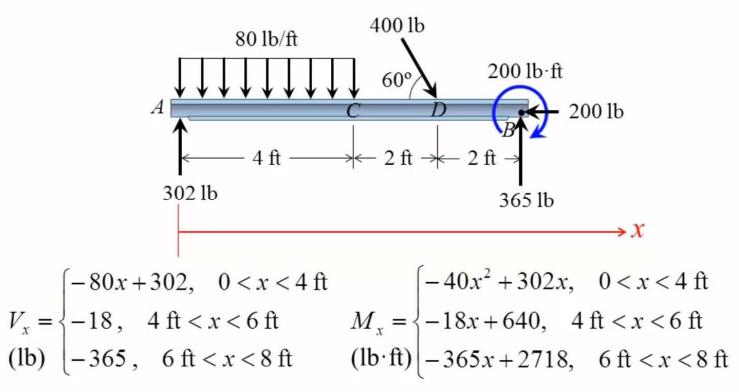
## Internal forces: Shear fore and bending moment functions and diagrams

#### **Section 3:**



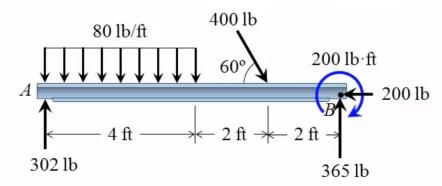
$$\begin{split} \sum F_y &= 302 - 320 - 400 \cdot \sin 60^\circ - V_x = 0 \\ \sum M_x &= -302 \cdot x + 320 \cdot (x - 2) \\ 400 \cdot \sin 60^\circ \cdot (x - 6) + M_x &= 0 \end{split} \qquad \vdots \qquad \begin{cases} V_x &= -365 \text{ (lb)} \\ M_B &= -365x + 2718 \text{ (lb · ft)} \end{cases} \end{split}$$

## Internal forces: Shear fore and bending moment functions and diagrams



# **Internal forces: Shear fore and bending moment functions and diagrams**

Representation 'Graph':



#### Shear force diagram

#### Bending moment diagram

